

APPENDIX G

DRAINAGE STUDY

**DRAINAGE STUDY
for
IONE BAND OF MIWOK INDIANS
CASINO PROJECT**

Prepared for:

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DRAINAGE STUDY
FOR
IONE BAND OF MIWOK INDIANS CASINO PROJECT

The Ione Band of Miwok Indians Casino Project encompasses approximately 216 acres, located on the east side of Highway 49, south of the City of Plymouth, in Amador County, California. The majority of the site is currently undeveloped, although there is an established commercial strip adjacent to the Highway.

The project area contributes to two distinct watersheds. Approximately 101 acres in the central portion of the site consist of mildly sloping grassland with scattered trees and brush. This area drains from south to north into Little Indian Creek. An existing on-site pond provides limited detention for a portion of the runoff. Little Indian Creek has its origin in approximately 100 acres of off-site watershed area on the west side of Highway 49. The Creek crosses the highway onto the project, thence flows northerly in a channel that roughly parallels Highway 49. Site runoff enters Little Indian Creek at several locations. The combined flow crosses Highway 49 just north of the project. The eastern and southern project perimeters are more steeply sloped, and support growths of heavy brush and/or trees. The perimeter area can be divided into seven relatively small shed areas tributary to Dry Creek. The Dry Creek channel, flowing in a southeasterly direction, is located off-site, to the south of the project.

Details of proposed site development are unknown at the time this drainage study is being completed. For purposes of the analysis, it is assumed that slope constraints preclude development within the Dry Creek sheds. It is assumed that the project area within the Little Indian Creek watershed will be fully developed, and will utilize storm drain pipes to collect and convey runoff across the project area to the existing point of discharge of Little Indian Creek near the northern project boundary. On-site storm water detention is provided for the purpose of peak flow mitigation.

The drainage study was completed in adherence to current practices utilizing the following parameters:

- o Precipitation – Mean annual precipitation over the project area is estimated to be 30 inches. The 24-hour precipitation total for a 100-year event is 5.31". Based on shed elevations, the SCS Type 1 storm pattern was used.
- o Runoff Curve Numbers – Curve numbers (CN's) reflect soil type and land use within shed areas. Soils underlying the study area are classified in the Exchequer series, in hydrologic soil group D, characterized by high runoff potential and low infiltration. CN's used in the study are summarized in Table 1.
- o Times of Concentration / Lag Times – Times of concentration for pre-project conditions were computed using standard equations. Post-project

times of concentration were assumed to be 10 – 15 minutes. Lag times were estimated to be 0.6 x time of concentration.

- Runoff Computations – The graphical HEC-1 computer program was used to develop runoff hydrographs based on the SCS unit hydrograph method.

HEC-1 hydrograph computations represent pre- and post-project runoff due to a 100-year storm. The study area encompasses only the area draining to Little Indian Creek, which was divided into on-site sheds 8, 9, 10, 11, and 12 as shown on the Exhibit. Shed W encompasses the off-site headwater area west of Highway 49. The convergence point for these sheds, the key point in the analyses, was assumed to occur where little Indian Creek crosses the north boundary of the project area.

Project build-out is assumed to result in impervious coverage of 90% of the area within Sheds 8-12. The remaining 10% of each shed is assumed to consist of landscaped area, resulting in a composite runoff curve number (CN) of 96 representing the post-project condition in each shed. Since storm drain layouts are not yet determined, times of concentration of 10 minutes or 15 minutes were assumed.

Detailed derivations of the Hec-1 input parameters, summarized in Table 1, are included in the Technical Appendix.

Table 1 – HEC-1 Input Parameters

SHED	AREA (sq mi)	PRE-PROJECT		POST-PROJECT	
		CN	LAG (hrs)	CN	LAG (hrs)
W	0.1563	74	0.255	74	0.255
8	0.0419	78	0.204	96	0.15
9	0.0578	78	0.183	96	0.15
10	0.0205	78	0.159	96	0.15
11	0.0209	80	0.124	96	0.10
12	0.0169	85	0.150	96	0.15

Detention storage for the purpose of post-project peak flow reduction is provided at the existing pond location. The goal of detention design is to limit releases so that flow at the northern project boundary, where detention outflow converges with flow in Little Indian Creek, is at or below the pre-project level. Runoff from sheds 8 and 9 is routed through the detention facility. The analyses indicate that modifications to the existing pond can result in a stage-storage-discharge configuration adequate to provide mitigation of increased runoff resulting from development within sheds 8-12. These results are summarized in Table 2. The complete HEC-1 runs are included in the Technical Appendix

Table 2 – HEC-1 Output Summary

Location	100-YEAR RUNOFF	
	Pre-Project	Post-Project
Upper reach of Little Indian Creek @ Confluence of Sheds W & 11	139 cfs	146 cfs
Middle reach of Little Indian Creek @ Confluence with Shed 10	155 cfs	177 cfs
Detention Inflow 1/	42	173 cfs
Detention Outflow 2/	41	62 cfs
Little Indian Creek @ project boundary (Confluence of detention outflow & Shed 12 with flow in main channel)	258 cfs	257 cfs

1/ Shed 8 runoff

2/ Combined runoff from Sheds 8 & 9

Hydrograph routing computations, summarized in Table 2 and included in detail in the Technical Appendix, show that increased runoff resulting from development of the Lone Band of Miwok Indians Casino Project according to assumptions outlined herein can be mitigated by means of on-site detention storage.

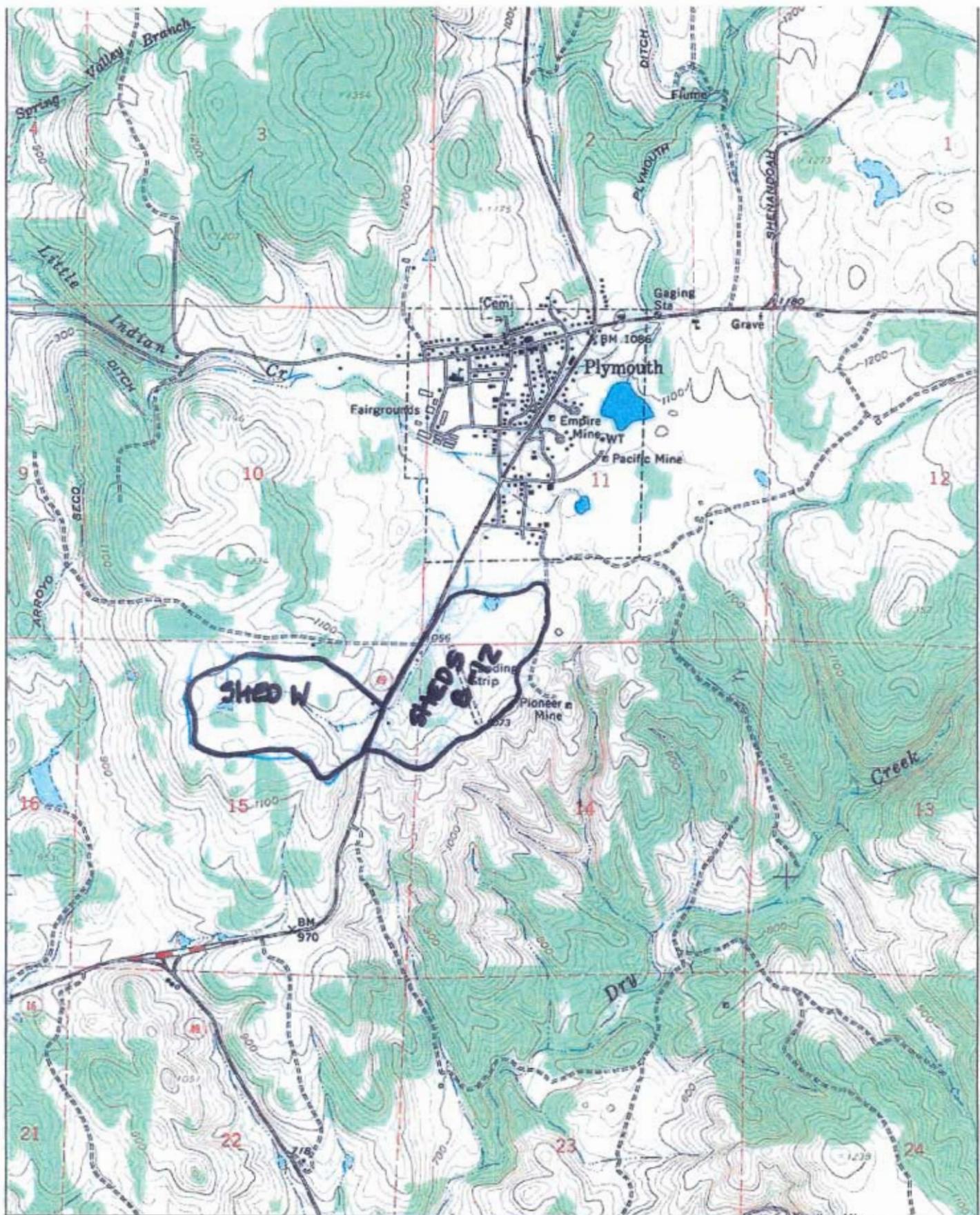
RESOURCES

Amador County Guidelines for Grading and Erosion Control, Amador County Public Works Agency, issued 11-21-03.

Erosion & Sediment Control Guidelines for Developing Areas of the Sierra Foothills and Mountains, High Sierra RC&D Council, October 1991.

Soil Survey Amador Area, California, United States Department of Agriculture Soil Conservation Service in cooperation with California Agricultural Experiment Station, August 1993.

TECHNICAL APPENDIX



TN * MN
15°

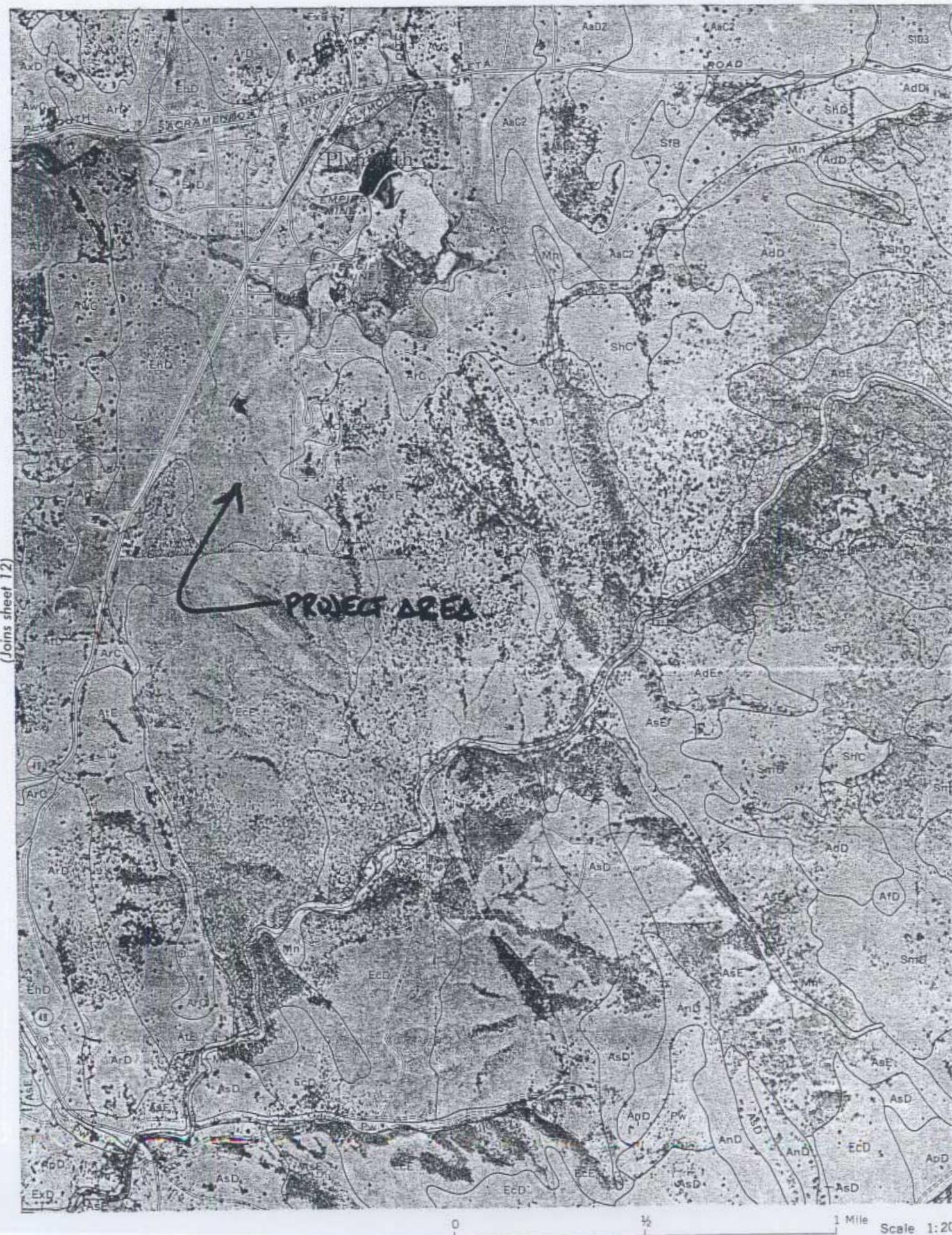
0 1000 FEET 0 500 1000 METERS

Map created with TOPO!® ©2002 National Geographic (www.nationalgeographic.com/topo)

SHED MAP - IONE BAND OF MINOK INDIANS CASINO PROJECT

SCS SOIL SURVEY

AMADOR AREA, CALIFORNIA



IONE BAND OF MIWOK INDIANS CASINO PROJECT EIS
DERIVATION OF SHED PARAMETERS USED IN HYDROLOGIC COMPUTATIONS - PRE-PROJECT CONDITIONS

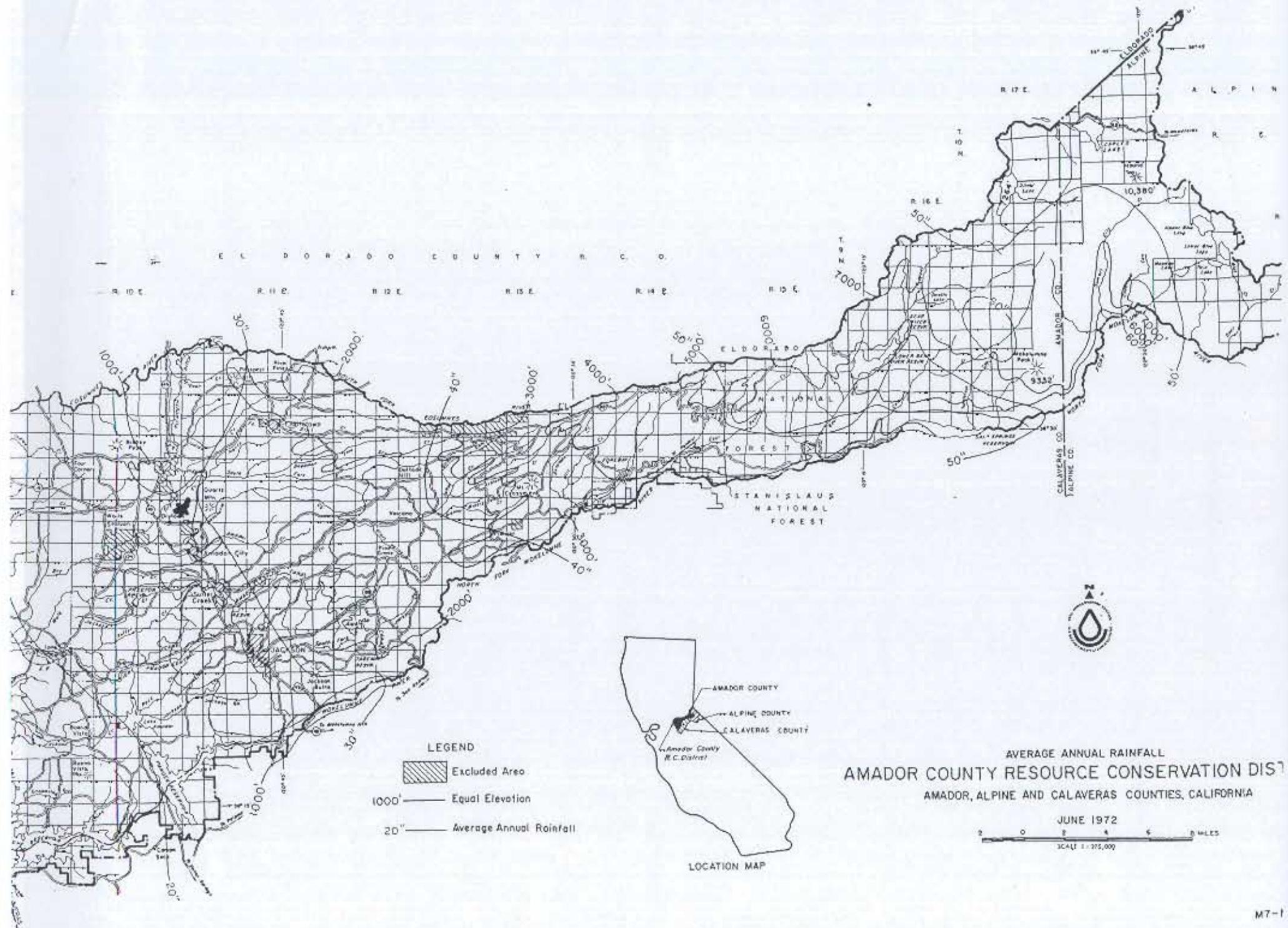
SHED #	AREA (ac)	L1 (ft)	H1up (ft)	H1dwn (ft)	S1 (ft/ft)	P (in)	n	Tt1 (hr)	L2 (ft)	H2up (ft)	H2dwn (ft)	S2 (ft/ft)	V2 (ft/sec)	Tt2 (hr)	SUM Tt (hr)	LAG (hr)	CN
W	100	300	*	*	0.067	30	0.3	0.14	3200	*	*	0.025	2.55	0.35	0.42	0.255	74
1	7.6	300	1153	1065	0.293	30	0.3	0.08	900	1065	970	0.106	5.24	0.05	0.14	0.081	78
2	6.7	300	1155	1093	0.207	30	0.3	0.09	700	1093	1002	0.130	5.82	0.03	0.13	0.076	78
3	2.7	300	1154	1100	0.180	30	0.3	0.09	400	1100	1057	0.108	5.29	0.02	0.12	0.073	78
4	15.4	300	1156	1095	0.203	30	0.35	0.10	1100	1095	885	0.191	7.05	0.04	0.15	0.093	73
5	45.4	300	1062	1015	0.157	30	0.35	0.11	2000	1015	805	0.105	5.23	0.11	0.28	0.168	73
6	21.3	300	1080	1035	0.150	30	0.6	0.17	3000	1035	790	0.082	4.61	0.18	0.36	0.213	77
7	10.4	300	1110	1085	0.083	30	0.45	0.17	1000	1085	975	0.110	5.35	0.05	0.22	0.130	77.5
8	26.8	300	1078	1065	0.043	30	0.3	0.16	1200	1065	1050	0.013	1.80	0.18	0.34	0.204	78
9	37	300	1094	1079	0.050	30	0.3	0.16	2200	1079	1045	0.015	2.01	0.30	0.30	0.183	78
10	13.1	300	1099	1073	0.087	30	0.3	0.12	1100	1073	1053	0.018	2.18	0.14	0.26	0.159	78
11	13.4	300	1143	1105	0.127	30	0.3	0.11	1000	1105	1075	0.030	2.79	0.10	0.21	0.124	80
12	10.8	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	85

FORMULAS:

$$Tt1 = [0.007(nL)^{0.8}]/P^{0.5}S1^{0.4}$$

$$V2 = 16.1345S2^{0.5}$$

$$Tt2 = L2/V2/60$$



PRE-PROJECT

HBC1 S/N: 1343001909 HMVersion: 6.33 Date File: C:\WINDOWS\TEMP\vbh0C5B.TMP
C:\HAESTAD\GHEC1\SAMPLE\PLYPMPRE.OUT

FLOOD HYDROGRAPH PACKAGE (HEC-1)
MAY 1991
VERSION 4.0.1E
RUN DATE 01/17/2004 TIME 20:18:34

U.S. ARMY CORPS OF ENGINEERS
HYDROLOGIC ENGINEERING CENTER
609 SECOND STREET
DAVIS, CALIFORNIA 95616
(916) 756-1104

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Full Microcomputer Implementation
by
Haestad Methods, Inc.

37 Brookside Road • Waterbury, Connecticut 06708 • (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -ANEXX- ON HM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREACK OUTFLOW SUMMERGENCY, SINGLE EVENT DAMAGE CALCULATION, DSS-WRITE STAGE FREQUENCY,
DSS-READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE, NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT PAGE 1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID 100-YEAR PRE-PROJECT RUNOFF FOR NORTH AREA
2 IT 5 310
3 IO 5 0
4 KK SHED W
5 KM OFFSITE AREA, W. OF HWY 49
6 KO 22
7 BA 0.1563
8 PB 5.31
9 IN 6
10 PC 0.0000 0.00174 0.00348 0.00522 0.00697 0.00871 0.01046 0.01220 0.01395 0.01570
11 PC 0.0174 0.01920 0.02095 0.02270 0.02446 0.02621 0.02797 0.02972 0.03148 0.03324
12 PC 0.0350 0.03677 0.03858 0.04041 0.04237 0.04416 0.04608 0.04803 0.05001 0.05201
13 PC 0.0540 0.05611 0.05821 0.06033 0.06248 0.06466 0.06687 0.06911 0.07138 0.07367
14 PC 0.0760 0.07835 0.08070 0.08307 0.08545 0.08784 0.09024 0.09265 0.09507 0.09751
15 PC 0.1000 0.10241 0.10487 0.10735 0.10984 0.11234 0.11485 0.11737 0.11990 0.12245
16 PC 0.1250 0.12761 0.13034 0.13317 0.13610 0.13915 0.14230 0.14557 0.14894 0.15241
17 PC 0.1560 0.15966 0.16334 0.16706 0.17082 0.17460 0.17842 0.18226 0.18614 0.19006
18 PC 0.1940 0.19817 0.20275 0.20775 0.21317 0.21900 0.22523 0.23185 0.23885 0.24623
19 PC 0.2540 0.26233 0.27139 0.28119 0.29173 0.30300 0.31942 0.34542 0.38784 0.46316

Page 1

C:\HABSTAD\GHEC1\SAMPLE\FLYMPRE.OUT
 20 PC 0.5150 0.53220 0.54760 0.56120 0.57300 0.58300 0.59188 0.60032 0.60832 0.61588
 21 PC 0.6230 0.62982 0.63648 0.64298 0.64932 0.65550 0.66152 0.66738 0.67308 0.67862
 22 PC 0.6840 0.68925 0.69440 0.69945 0.70440 0.70925 0.71400 0.71865 0.72320 0.72765
 23 PC 0.7320 0.73625 0.74040 0.74445 0.74840 0.75225 0.75600 0.75965 0.76320 0.76665
 24 PC 0.7700 0.77329 0.77658 0.77981 0.78304 0.78625 0.78944 0.79261 0.79576 0.79889
 25 PC 0.8020 0.80509 0.80816 0.81121 0.81424 0.81725 0.82024 0.82321 0.82616 0.82909
 26 PC 0.8320 0.83489 0.83776 0.84061 0.84344 0.84625 0.84904 0.85181 0.85456 0.85729
 27 PC 0.8600 0.86269 0.86536 0.86801 0.87064 0.87325 0.87584 0.87841 0.88096 0.88349
 28 PC 0.8860 0.88849 0.89096 0.89341 0.89584 0.89825 0.90064 0.90301 0.90536 0.90769
 29 PC 0.9100 0.91229 0.91456 0.91681 0.91904 0.92125 0.92344 0.92561 0.92776 0.92989
 30 PC 0.9320 0.93409 0.93616 0.93821 0.94024 0.94225 0.94424 0.94621 0.94816 0.95009
 31 PC 0.9520 0.95389 0.95576 0.95761 0.95944 0.96125 0.96304 0.96481 0.96656 0.96829
 32 PC 0.9700 0.97169 0.97336 0.97501 0.97664 0.97825 0.97984 0.98141 0.98296 0.98449
 33 PC 0.9860 0.98749 0.98896 0.99041 0.99184 0.99325 0.99464 0.99561 0.99736 0.99869
 34 PC 1.0000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000
 35 LS 74
 36 UD 0.255
 37 KK SHED11
 38 KM
 39 KO 22
 40 BA .0209
 41 LS 80
 42 UD 0.124
 43 KK SUM1
 44 KM
 45 KO 22
 46 HC 2
 HEC-1 INPUT

PAGE 2

LINE ID.....1.....3.....3.....4.....5.....6.....7.....8.....9.....10

47 KK RTE1
 48 KM ESTIMATED SECTION
 49 KO 22
 50 RD 1400 .01 .035 TRAP 5 3
 51 KK SHED10
 52 KM
 53 KO 22
 54 BA .0205
 55 LS 78
 56 UD 0.159
 57 KK SUM2
 58 KM
 59 KO 22
 60 HC 2
 61 KK RTE2
 62 KM ESTIMATED SECTION
 63 KO 22
 64 RD 1100 .008 .035 TRAP 5 3
 65 KK SHED8
 66 KM
 67 KO 22
 68 BA .0419
 69 LS 78
 70 UD .204
 71 KK POND1
 72 KM ROUTE THROUGH EX. ON-SITE POND
 73 KO 22
 74 RS 1 ELEV 1047.7
 75 SA .063 0.33 0.58
 76 SE 1047.7 1050 1051
 77 SL 1045 .01 0.6 0.5
 78 SS 1050.5 125 3 1.5
 79 KK SHED 9
 80 KM
 81 KO 22
 82 BA .0578
 83 LS 78

Page 2

C:\RAESTAD\GHEC1\SAMPLE\PLYMPRE.OUT

84 UD 0.183
85 KK SHED12
86 KM
87 KO 22
88 EA .0169
89 LS .85
90 UD .15

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

91 KK BNDY
92 KM COMBINED RUNOFF & APPROX. PROJECT BOUNDARY
93 KO 22
94 NC 4
95 ZZ

HEC1 S/N: 1343001909 HMVersion: 6.33 Data File:
C:\WINDOWS\TEMP\~vbbh0C5B.TMP

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/17/2004 TIME 20:18:34 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

100-YEAR PRE-PROJECT RUNOFF FOR NORTH AREA

3 TO OUTPUT CONTROL VARIABLES
IPRINT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
INMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 310 NUMBER OF HYDROGRAPH ORDINATES
MDATE 2 0 ENDING DATE
MOTIME 0145 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
TOTAL TIME BASE 25.75 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

4 KK * SHED W *

6 KO OUTPUT CONTROL VARIABLES
IPRINT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL

C:\HAESTAD\GHEC1\SAMPLE\FLYMPRE.OUT
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

37 KK * SHED11 *

39 EO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

43 KK * SUM1 *

45 EO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

47 KK * RTE1 *

49 EO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

51 KK * SHED10 *

53 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPILOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

57 KK * SUM2 *

59 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPILOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

61 KK * RTB2 *

63 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPILOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

65 KK * SHED# *

67 KO OUTPUT CONTROL VARIABLES

C:\HAESTAD\GHECI\SAMPLE\PLYMPRE.OUT

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

71 KK * POND1 *

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

73 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

79 KK * SHED 9 *

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

81 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

85 KK * SHED13 *

*** * *** * *** * *** * *** * *** * *** * *** * *** * *** * ***

87 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

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*****
*   BNDY *
*****

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93 KD OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK 6-HOUR	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
24-HOUR	72-HOUR								
HYDROGRAPH AT SHED 8	120.	10.17	28.	11.	10.	0.16			
HYDROGRAPH AT SHED11	26.	10.00	5.	2.	2.	0.02			
2 COMBINED AT SUM1	139.	10.08	33.	13.	12.	0.18			
ROUTED TO RTE1	137.	10.17	33.	13.	12.	0.18			
HYDROGRAPH AT SHED10	22.	10.08	4.	2.	2.	0.02			
2 COMBINED AT SUM2	155.	10.17	37.	14.	13.	0.20			
ROUTED TO RTE2	150.	10.25	37.	14.	13.	0.20			
HYDROGRAPH AT SHED8	42.	10.08	9.	3.	3.	0.04			
ROUTED TO POND1	41.	10.08	8.	3.	3.	0.04			
1050.73	10.08								
HYDROGRAPH AT SHED 9	60.	10.08	12.	5.	4.	0.06			
HYDROGRAPH AT SHED12	23.	10.00	4.	2.	2.	0.02			
4 COMBINED AT BNDY	258.	10.17	61.	24.	22.				
0.31									

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-Cunge ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	INTERPOLATED TO COMPUTATION INTERVAL			VOLUME
						(MIN)	(CFS)	(MIN)	
RTE1	MANE	4.02	138.35	610.45	2.68	5.00	137.43	610.00	2.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2531E+02 EXCESS=0.0000E+00 OUTFLOW=0.2532E+02 BASIN STORAGE=0.8775E-03 PERCENT ERROR= 0.0

RTE2	MANE	3.32	151.57	611.07	2.71	5.00	150.27	615.00	2.71
------	------	------	--------	--------	------	------	--------	--------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2857E+02 EXCESS=0.0000E+00 OUTFLOW=0.2857E+02 BASIN STORAGE=0.9837E-03 PERCENT ERROR= 0.0

C:\HAESTAD\GHEC1\SAMPLE\PLYMPRE.CUT

*** NORMAL END OF HEC-1 ***

POST-PROJECT

C:\HAESTAD\GHEC1\SAMPLE\PLYMPST.OUT
HEC1 S/N: 1343001909 HWVersion: 6.33 Data File: C:\WINDOWS\TEMP\VDRH06F.TMP

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/24/2004 TIME 15:55:35 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXXX XXXXX X XXXXX X
X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX

		Full Microcomputer Implementation		
		by		
		Haestad Methods, Inc.		

37 Brookside Road • Waterbury, Connecticut 06708 • (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1EW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD HAS BEEN CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION.
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE, GREEN AND AMPT INFILTRATION
KINEMATIC WAVE, NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT										PAGE 1	
LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10										
1	ID 100-YEAR POST-PROJECT RUNOFF (NORTH AREA)										
2	IT 5										
3	IC 5 0										
4	KK SHED W										
5	KM OFFSITE AREA, W. OF HWY 48										
6	KO										
7	BA 0.1563										
8	PB 5.31										
9	IN 6										
10	PC 0.0000 0.00174 0.00348 0.00522 0.00697 0.00871 0.01046 0.01220 0.01395 0.01570										
11	PC 0.0174 0.01920 0.02095 0.02270 0.02446 0.02621 0.02797 0.02972 0.03148 0.03324										
12	PC 0.0350 0.03677 0.03858 0.04041 0.04227 0.04416 0.04608 0.04803 0.05001 0.05201										
13	PC 0.0540 0.05611 0.05821 0.06033 0.06248 0.06466 0.06687 0.06911 0.07138 0.07367										
14	PC 0.0760 0.07835 0.08070 0.08307 0.08545 0.08784 0.09024 0.09265 0.09507 0.09751										
15	PC 0.1000 0.10241 0.10487 0.10735 0.10984 0.11234 0.11485 0.11737 0.11990 0.12245										
16	PC 0.1250 0.12761 0.13034 0.13317 0.13610 0.13915 0.14230 0.14557 0.14894 0.15241										
17	PC 0.1560 0.15966 0.16334 0.16706 0.17082 0.17460 0.17842 0.18226 0.18614 0.19006										
18	PC 0.1940 0.19817 0.20275 0.20775 0.21317 0.21900 0.22523 0.23185 0.23885 0.24623										
19	PC 0.2540 0.26233 0.27139 0.28119 0.29173 0.30300 0.31942 0.34542 0.38784 0.46316										

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```

20 PC 0.5150 0.53220 0.54760 0.56120 0.57300 0.58300 0.59188 0.60032 0.60832 0.61588
21 PC 0.6230 0.62982 0.63648 0.64298 0.64932 0.65550 0.66152 0.66738 0.67308 0.67862
22 PC 0.6840 0.68925 0.69440 0.69945 0.70440 0.70925 0.71400 0.71865 0.72320 0.72765
23 PC 0.7320 0.73625 0.74040 0.74445 0.74840 0.75225 0.75600 0.75965 0.76320 0.76665
24 PC 0.7700 0.77329 0.77656 0.77981 0.78304 0.78625 0.78944 0.79261 0.79576 0.79889
25 PC 0.8020 0.80505 0.80816 0.81121 0.81424 0.81725 0.82024 0.82321 0.82616 0.82909
26 PC 0.8320 0.83489 0.83776 0.84061 0.84344 0.84625 0.84904 0.85181 0.85456 0.85729
27 PC 0.8800 0.88269 0.88536 0.88680 0.87064 0.87325 0.87584 0.87841 0.88096 0.88349
28 PC 0.9860 0.88849 0.89096 0.89341 0.89584 0.89825 0.90064 0.90301 0.90538 0.90765
29 PC 0.9100 0.91229 0.91456 0.91681 0.91904 0.92125 0.92344 0.92561 0.92776 0.92989
30 PC 0.9320 0.93409 0.93616 0.93821 0.94024 0.94235 0.94424 0.94621 0.94816 0.95009
31 PC 0.9520 0.95389 0.95576 0.95761 0.95944 0.96125 0.96304 0.96481 0.96656 0.96829
32 PC 0.9700 0.97169 0.97336 0.97501 0.97664 0.97825 0.97984 0.98141 0.98296 0.98449
33 PC 0.9860 0.98749 0.98896 0.99041 0.99184 0.99325 0.99464 0.99601 0.99736 0.99869
34 PC 1.0000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000
35 LS 74
36 UD 0.255
37 KK SHED11
38 KM POST-PROJECT; ASSUME 90% IMPERVIOUS; 10 MIN. TIME OF CONCENTRATION
39 KO 22
40 BA .0209
41 LS 96
42 UD 0.1
43 KK SUM1
44 KM
45 KO 22
46 HC 2
      HEC-1 INPUT

```

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

47 KK RTE1
48 KM ESTIMATED SECTION
49 KO 22
50 RD 1400 .01 .035 TRAP 5 3
51 KK SHED10
52 KM POST-PROJECT; ASSUME 90% IMPERVIOUS; 10 MIN. TIME OF CONCENTRATION
53 KO 22
54 BA .0205
55 LS 96
56 UD 0.1
57 KK SUM2
58 KM
59 KO 22
60 HC 2
61 KK RTE2
62 KM ESTIMATED SECTION
63 KO 22
64 RD 1100 .008 .035 TRAP 5 3
65 KK SHED8
66 KM POST-PROJECT; ASSUME 90% IMPERVIOUS; 15 MIN. TIME OF CONCENTRATION
67 KO 22
68 BA .0419
69 LS 96
70 UD .15
71 KK SHED 9
72 KM POST- PROJECT; ASSUME 90% IMPERVIOUS; 15 MIN. TIME OF CONCENTRATION
73 KO 22
74 BA .0578
75 LS 96
76 UD 0.15
77 KK SHED12
78 KM POST-PROJECT; ASSUME 90% IMPERVIOUS; 15 MIN. TIME OF CONCENTRATION
79 KO 22
80 BA .0169
81 LS 96
82 UD .15

```

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83 KK BNDY
84 RM COMBINED RUNOFF & APPROX. PROJECT BOUNDARY
85 KO 22
86 HC 4
87 ZZ

.EC1 S/N: 1343001909 HMVersion: 6.33 Data File:
C:\WINDOWS\TEMP\vbh106F.TMP

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/14/2004 TIME 15:55:35 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

100-YEAR POST-PROJECT RUNOFF (NORTH AREA)

3 IO OUTPUT CONTROL VARIABLES
IPRINT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
INMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 310 NUMBER OF HYDROGRAPH ORDINATES
HDDATE 2 0 ENDING DATE
HDTIME 0145 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
TOTAL TIME BASE 25.75 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

4 KK SHED W

6 KO OUTPUT CONTROL VARIABLES
IPRINT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

37 KK * SHED11 *

39 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPUNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

43 KK * SUM1 *

45 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPUNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

47 KK * RTE1 *

49 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPUNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	310	LAST ORDINATE PUNCHED OR SAVED
TIMINT	0.083	TIME INTERVAL IN HOURS

51 KK * SHED10 *

53 KO OUTPUT CONTROL VARIABLES

IPRINT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL

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QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

57 KK * SUM2 *

59 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

61 KK * RTE2 *

63 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

65 KK * SHED8 *

67 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

71 KK * SHED 9 *

73 KO OUTPUT CONTROL VARIABLES
 IPRT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

77 KK * SHED12 *

79 KO OUTPUT CONTROL VARIABLES
 IPRT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

83 KK * BNDY *

85 KO OUTPUT CONTROL VARIABLES
 IPRT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

		OPERATION	STATION	PEAK FLOW	TIME OF PEAK 6-HOUR	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
24-HOUR	72-HOUR								
	HYDROGRAPH AT SHED W			120.	10.17	28.	11.	10.	0.16
	HYDROGRAPH AT SHED11			40.	10.00	7.	3.	3.	0.02
	2 COMBINED AT SUM1			145.	10.08	35.	14.	13.	0.18

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ROUTED TO RTE1	145.	10.17	35.	14.	13.	0.18
HYDROGRAPH AT SHED10	40.	10.00	7.	3.	2.	0.02
2 COMBINED AT SUM2	167.	10.08	41.	16.	15.	0.20
ROUTED TO RTE2	165.	10.17	41.	16.	15.	0.20
HYDROGRAPH AT SHED8	73.	10.00	13.	5.	5.	0.04
HYDROGRAPH AT SHED 8	100.	10.00	18.	8.	7.	0.06
HYDROGRAPH AT SHED12	29.	10.00	5.	2.	2.	0.02
4 COMBINED AT ENDY	351.	10.08	78.	32.	29.	

0.31.

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
RTE1	MANE	3.96	145.63	605.47	2.88	5.00	144.82	610.00	2.88

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2718E+02 EXCESS=0.0000E+00 OUTFLOW=0.2718E+02 BASIN STORAGE=0.1137E-02 PERCENT ERROR= 0.0

RTE2	MANE	3.26	165.80	608.78	3.08	5.00	165.32	610.00	3.08
------	------	------	--------	--------	------	------	--------	--------	------

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3248E+02 EXCESS=0.0000E+00 OUTFLOW=0.3248E+02 BASIN STORAGE=0.7374E-03 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

POST-PROJECT W/ DETENTION

HEC1 S/N: 1343001909 MMVersion: 6.33 Data File: C:\WINDOWS\TEMP\~vhb3C65.TMP

```
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/24/2004 TIME 15:45:05 *
*****
```

```
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
```

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X   X   XXXXXX   XXXXX   X
X   X   X   X   XX
X   X   X
XXXXXX XXXX   X   XXXXX X
X   X   X   X
X   X   X   X   X
X   X   XXXXXX   XXXXX   XXX
```

```
::::: Full Microcomputer Implementation :::::
      by
      Haestad Methods, Inc.
:::::
```

17 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE, NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT										PAGE 1
LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10									
1	ID 100-YEAR POST-PROJECT RUNOFF FOR NORTH AREA W/ DET									
2	IT 5	310								
3	IO 5	0								
4	KK SHED W									
5	KK OFFSITE AREA, N. OF HWY 48									
6	KD	22								
7	BA 0.1563									
8	PR 5.31									
9	IN 6									
10	PC 0.0000 0.00174 0.00348 0.00522 0.00697 0.00871 0.01046 0.01220 0.01395 0.01570									
11	PC 0.0174 0.01920 0.02095 0.02270 0.02446 0.02621 0.02797 0.02972 0.03148 0.03324									
12	PC 0.0350 0.03677 0.03858 0.04041 0.04227 0.04416 0.04608 0.04803 0.05001 0.05201									
13	PC 0.0540 0.05611 0.05821 0.06033 0.06248 0.06466 0.06687 0.06911 0.07138 0.07367									
14	PC 0.0760 0.07835 0.08070 0.08307 0.08545 0.08784 0.09024 0.09265 0.09507 0.09751									
15	PC 0.1000 0.10241 0.10487 0.10735 0.10984 0.11234 0.11485 0.11737 0.11990 0.12245									
16	PC 0.1250 0.12761 0.13034 0.13317 0.13610 0.13915 0.14230 0.14557 0.14894 0.15241									
17	PC 0.1560 0.15966 0.16334 0.16706 0.17082 0.17460 0.17842 0.18226 0.18614 0.19006									
18	PC 0.1940 0.19817 0.20275 0.20775 0.21317 0.21900 0.22523 0.23185 0.23885 0.24623									
19	PC 0.2540 0.26233 0.27139 0.28119 0.29173 0.30300 0.31942 0.34542 0.38784 0.46316									

Page 1

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```

20      PC  0.5150 0.53220 0.54760 0.56120 0.57300 0.58300 0.59188 0.60032 0.60832 0.61588
21      PC  0.6230 0.62982 0.63648 0.64298 0.64932 0.65550 0.66152 0.66718 0.67308 0.67862
22      PC  0.6840 0.68925 0.69440 0.69945 0.70440 0.70925 0.71400 0.71865 0.72320 0.72765
23      PC  0.7320 0.73625 0.74040 0.74445 0.74840 0.75225 0.75600 0.75965 0.76320 0.76665
24      PC  0.7700 0.77329 0.77656 0.77981 0.78304 0.78625 0.78944 0.79261 0.79576 0.79889
25      PC  0.8020 0.80509 0.80816 0.81121 0.81424 0.81725 0.82024 0.82321 0.82616 0.82909
26      PC  0.8320 0.83489 0.83776 0.84061 0.84344 0.84625 0.84904 0.85181 0.85456 0.85729
27      PC  0.8600 0.86269 0.86536 0.86801 0.87064 0.87325 0.87584 0.87841 0.88096 0.88349
28      PC  0.8860 0.88849 0.89096 0.89341 0.89584 0.89825 0.90064 0.90301 0.90536 0.90769
29      PC  0.9100 0.91229 0.91458 0.91681 0.91904 0.92125 0.92344 0.92561 0.92776 0.92989
30      PC  0.9320 0.93409 0.93616 0.93821 0.94024 0.94225 0.94424 0.94621 0.94816 0.95009
31      PC  0.9520 0.95389 0.95576 0.95761 0.95944 0.96125 0.96304 0.96481 0.96656 0.96829
32      PC  0.9700 0.97169 0.97336 0.97501 0.97664 0.97825 0.97984 0.98141 0.98296 0.98449
33      PC  0.9860 0.98749 0.98896 0.99041 0.99184 0.99325 0.99464 0.99601 0.99736 0.99869
34      PC  1.0000 1.00000 1.00000 1.00000 1.00000 1.00000
35      LS    74
36      UD    0.255

```

```

37      KK  SHED11
38      KM
39      KO
40      BA  .0205          22
41      LS   96
42      UD   0.1
43      KK  SUM1
44      KM
45      KO
46      HC   2          22

```

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

47      KK  RTE1
48      KM  ESTIMATED SECTION - USE 36" HDPE
49      KO
50      RD  1400  .01  .013          CIRC   3          22
51      KK  SHED10
52      KM
53      KO
54      BA  .0205          22
55      LS   96
56      UD   0.15
57      KK  SUM2
58      KM
59      KO
60      HC   2          22
61      KK  RTE2
62      KM  ESTIMATED SECTION - USE 36" HDPE
63      KO
64      RD  1100  .008  .013          CIRC   3          22
65      KK  SHED8
66      KM
67      KO
68      BA  .0419          22
69      LS   96
70      UD   .15
71      KK  SHED 9
72      KM
73      KO
74      BA  .0578          22
75      LS   96
76      UD   0.15
77      KK  Node15
78      KM
79      KO
80      HC   2          22
81      KK  DET1
82      KM  DETENTION ROUTING; MODIFY EX. POND; F.L. OUT 1045; C.L. OUT 1046.5; 6 SQ. FT.

```

C:\HAEESTAD\GHEC1\SAMPLE\PLYMD1.OUT

```

83      EO          22
84      RS      1    STOR      0
85      SA      .1    1.3     1.5    1.85
86      SE    1045   1050   1051  1051.5
87      SL   1046.5    6     .6     .5
88      SG   1051.3    20     3     1.5
89      HEC-1 INPUT

```

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

89      KK SHED12
90      KM
91      KO          22
92      BA   ,0169
93      LS      56
94      UD   ,15
95      KK BNDY
96      KM COMBINED RUNOFF & APPROX. PROJECT BOUNDARY
97      KO          22
98      HC      3
99      ZZ

```

EC1 S/N: 1343001909 RMVersion: 6.33 Data File:
C:\WINDOWS\TEMP\vhb3C65.TMP

```
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/24/2004 TIME 15:45:05 *
*****
```

```
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
```

100-YEAR POST-PROJECT RUNOFF FOR NORTH AREA W/ DET

3 10 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 310 NUMBER OF HYDROGRAPH ORDINATES
 NEDATE 2 0 ENDING DATE
 EDTIME 0145 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 25.75 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

* KK SHED W *

 6 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

37 KK

* SHED11 *

39 KO

 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

43 KK

* SUM1 *

45 KO

 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

47 KK

* RTE1 *

49 KO

 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED

C:\HAESTAD\GHEC1\SAMPLE\PLYMD1.OUT
TIMINT 0.083 TIME INTERVAL IN HOURS

51 KK * SHED10 *

53 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

57 KK * SUM2 *

59 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

61 KK * RTE3 *

63 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

C:\VAESTAD\GHEC1\SAMPLE\PLVMDL.OUT
65 KK * SHED8 *

67 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

71 KK * SHED 9 *

73 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

77 KK * Node15 *

79 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

81 KK * DST1 *

83 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT

C:\RAESTAD\GHEC1\SAMPLE\PLYMD1.OUT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

 89 KK * SHED12 *

91 KO OUTPUT CONTROL VARIABLES
 IPENT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

 95 KK * BNDY *

97 KO OUTPUT CONTROL VARIABLES
 IPENT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 310 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK 6-HOUR	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
24-HOUR	72-HOUR						
HYDROGRAPH AT SHED 8	120.	10.17	28.	11.	10.	0.16	
HYDROGRAPH AT SHED11	40.	10.00	7.	3.	3.	0.02	
2 COMBINED AT SUM1	146.	10.08	35.	14.	13.	0.18	
ROUTED TO RTE1	142.	10.08	35.	14.	13.	0.18	
HYDROGRAPH AT SHED10	36.	10.00	7.	3.	3.	0.02	
2 COMBINED AT SUM2	177.	10.08	41.	16.	15.	0.20	
ROUTED TO RTE2	172.	10.08	41.	16.	15.	0.20	
HYDROGRAPH AT SHED8	73.	10.00	13.	5.	5.	0.04	
HYDROGRAPH AT SHED 9	100.	10.00	18.	8.	7.	0.06	
2 COMBINED AT Node15	173.	10.00	32.	13.	12.	0.18	

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ROUTED TO DET1	62.	10.42	32.	13.	12.	0.10
1051.06	10.42					
HYDROGRAPH AT SHED12	29.	10.00	5.	2.	2.	0.02
3 COMBINED AT RDY	257.	10.08	78.	31.	29.	
0.31						

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-Cunge ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	INTERPOLATED TO COMPUTATION INTERVAL			
						DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CPS)	(MIN)	(IN)	(MIN)	(CPS)	(MIN)	(IN)
RTE1	MANE	1.59	145.18	607.48	2.88	5.00	142.17	605.00	2.88

CONTINUITY SUMMARY (AC-PT) - INFLOW=0.2718E+02 EXCESS=0.0000E+00 OUTFLOW=0.2718E+02 BASIN STORAGE=0.1929E-03 PERCENT ERROR= 0.0

RTE2	MANE	1.32	174.96	606.69	3.08	5.00	171.90	605.00	3.08
------	------	------	--------	--------	------	------	--------	--------	------

CONTINUITY SUMMARY (AC-PT) - INFLOW=0.3248E+02 EXCESS=0.0000E+00 OUTFLOW=0.3248E+02 BASIN STORAGE=0.1512E-03 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

